



## Original Research Article

# Synthesis and Characterization of Nano-Palm Oil Fuel Ash and Nano-Fly Ash as Cementitious Fillers in High-Performance Engineering Cementitious Composites

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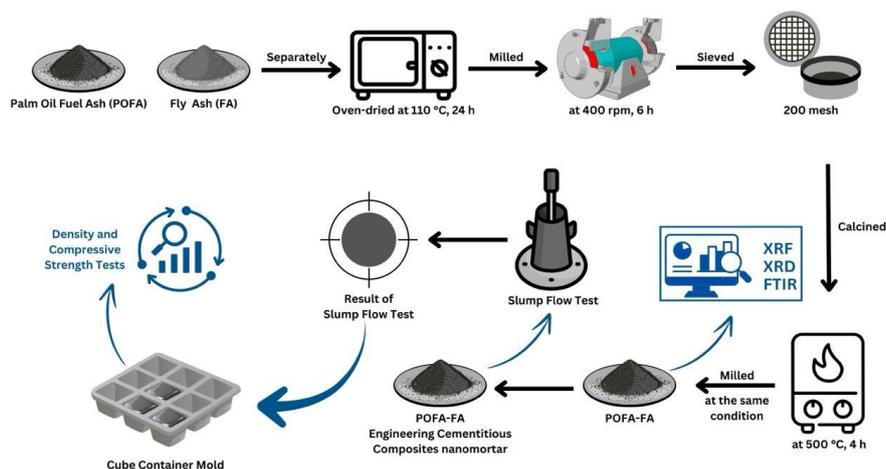
Pozzolanic reactivity

Environmental sustainability

## ABSTRACT

This study aims to enhance the pozzolanic reactivity and cementitious performance of palm oil fuel ash (POFA) and fly ash (FA) nanoparticles as eco-friendly supplementary materials in nanostructured engineered cementitious composites (ECC), thereby reducing industrial boiler waste and contributing to sustainable construction practices. POFA and FA were dried and ground into nanoparticles using a ball mill with replacement levels of 0%, 5%, 10%, and 15%. Characterization was performed using XRF, XRD, and FTIR, while physical tests included slump flow, density, and compressive strength. XRF confirmed the cementitious and pozzolanic properties of POFA and FA, with POFA containing over 50%  $\text{SiO}_2 + \text{Al}_2\text{O}_3 + \text{Fe}_2\text{O}_3$  and FA exceeding 70%. XRD showed average crystal sizes of 28.02 nm (POFA) and 16.46 nm (FA) with amorphous phases of 71.78% and 91.93%, respectively. FTIR revealed dominant Si-O-Si and Si-OH groups, indicating high silica content and pozzolanic reactivity. The slump flow decreased with higher POFA and FA content due to water absorption but remained within the high-performance concrete range (500–800 mm). Density values decreased yet stayed between normal and lightweight concrete limits. The compressive strength increased with higher nanoparticle content, reaching 46.34–72.73 MPa at 28 days, confirming improved hydration and mechanical performance.

## GRAPHICAL ABSTRACT



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## **Introduction**

In Indonesia, the primary fuels used in industrial boilers and power plants are coal and palm kernel shells. The combustion of these fuels generates large quantities of fly ash (FA) and palm oil fuel ash (POFA) as waste by-products. POFA typically contains a high percentage of silica oxide ( $\text{SiO}_2$ ), ranging from 48.5% to 61% [1-5].

Similarly, FA is recognized as a pozzolanic material [6-8], containing significant amounts of silica ( $\text{SiO}_2$ ), alumina ( $\text{Al}_2\text{O}_3$ ), iron oxide ( $\text{Fe}_2\text{O}_3$ ), and calcium oxide (CaO), along with smaller quantities of potassium, sodium, titanium, and sulfur [9-13].

These chemical compositions indicate that both POFA and FA possess strong cementitious potential, allowing them to react effectively within the cement matrix of Engineering Cementitious Composites (ECC), thereby producing high-performance concrete. ECC, also known as Strain Hardening Cement-Based Composites (SHCC) or more popularly as "bendable concrete" [14], is a type of fiber-reinforced material designed to overcome the brittleness of normal concrete. ECC exhibits a strain capacity of about 3-7%, approximately 300 times greater than that of conventional concrete, which is only around 0.01% [15,16].

This superior ductility results from its micromechanically engineered matrix and the inclusion of reinforcing fibers such as polyvinyl alcohol (PVA) or high-density polyethylene (HDPE). Previous studies on ECCs and strain-hardening cementitious composites (SHCCs) have demonstrated continuous advancements in micromechanical design, fiber optimization, and matrix innovation to enhance ductility, crack control, and environmental performance. Initially developed as fiber-reinforced composites exhibiting strain hardening and multiple microcracking at low fiber volumes [17-19], ECCs have evolved through the incorporation of synthetic fibers such as PVA and HDPE, and

hybrid systems using steel and polyethylene fibers to improve strength and energy absorption [19-21].

Concurrently, the cementitious matrix has transitioned from ordinary portland cement-based systems toward sustainable blends containing supplementary cementitious materials like FA, blast furnace slag, and limestone powder to improve durability, workability, and strain capacity [22-24].

These innovations have broadened ECC applications in structural and seismic resilience contexts, where their pseudo-ductile and self-healing behaviors enhance crack control and energy dissipation [25-27].

More recent trends emphasize eco-efficient ECCs utilizing industrial by-products to reduce carbon footprints while maintaining superior mechanical and durability performance [28,29].

Although extensive research on ECCs and SHCCs has explored fiber optimization, micromechanical design, and matrix enhancement to improve ductility and crack control, most studies have relied heavily on conventional cementitious materials such as Portland cement and FA. While these approaches have successfully advanced ECC performance, limited attention has been given to integrating alternative industrial by-products with high silica content, particularly POFA, despite its abundance and potential as a sustainable cementitious material. Furthermore, few studies have investigated the nanoparticle-scale modification of such by-products to enhance their pozzolanic reactivity and influence on ECC microstructure and mechanical properties. Therefore, this study aims to utilize POFA and FA as nanoparticle-based partial cement replacements to develop sustainable nanostructured ECCs with optimized cementitious reactions, improved compressive strength, and enhanced eco-efficiency. This approach not only contributes to high-performance concrete development, but also promotes value-added recycling, reduces air

pollution, conserves natural resources, and increases the economic value of industrial waste.

## Materials and Methods

### Materials

The POFA used in this study was obtained from the Tinjowan Palm Oil Plantation, located in Tinjowan Village, Simalungun Regency, Indonesia, while the FA was sourced from PT Socimas, Medan, North Sumatra, Indonesia. The other materials used in this study included Ordinary Portland Cement (OPC) as the primary binder, silica sand as fine aggregate, PVA fibres as reinforcement, water, and a high-range water-reducing admixture (superplasticizer) to improve workability.

### ECC nanomortar manufacturing

The initial stage of this study involved the preparation of materials, where POFA and FA were processed into nanoparticles using a ball mill. Before milling, both POFA and FA were oven-dried at 110 °C for 24 hours to remove moisture. The dried materials were then ground using a ball mill at 400 rpm for 6 hours, sieved through a 200-mesh sieve, and subsequently calcined at 500 °C for 4 hours. The calcined powders were then milled again for another 6 hours at 400 rpm to obtain uniform nanoparticle POFA and FA. The nanoparticle POFA and FA were characterized using X-ray fluorescence (XRF) to determine their oxide composition, X-ray diffraction (XRD) to analyze crystal structure and size, and Fourier transform infrared spectroscopy (FTIR) to identify functional groups. The nanomortar ECC mixtures were prepared using OPC as the main binder, silica sand as fine aggregate, PVA fibers as reinforcement, water, and a superplasticizer to enhance workability. The POFA and FA were used as partial cement replacements in varying proportions. Each material (POFA and FA) was

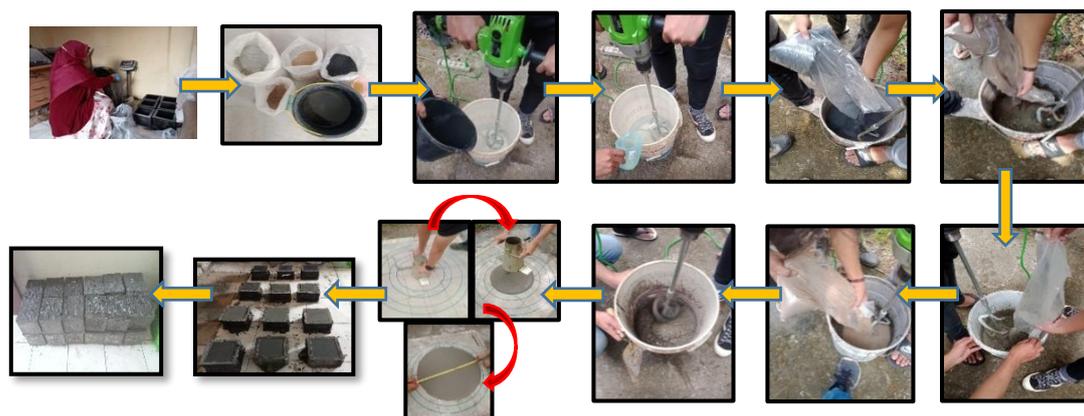
varied at 0%, 5%, 10%, and 15% by weight of cement, resulting in a total of 16 mix combinations, as shown in [Table 1](#).

**Table 1.** Matrix of POFA and FA percentage variations in ECC nanomortar mixtures

Composition	POFA			
FA	0%	5%	10%	15%
0%	S1	S5	S9	S13
5%	S2	S6	S10	S14
10%	S3	S7	S11	S15
15%	S4	S8	S12	S16

### Slump flow test

The slump flow test, a standard procedure to evaluate the flowability and workability of self-compacting or highly flowable concrete, was performed during the preparation of the POFA-FA ECC nanomortar. All constituent materials were prepared and weighed according to the designated mix proportions, and then mixed in a bucket using a drill mixer while recording the mixing time with a stopwatch. The total mixing duration was limited to 12 minutes to ensure uniform consistency. Before testing, all equipment was cleaned and dried. The Abrams cone was placed vertically on the base plate and held firmly to prevent movement. The POFA-FA ECC nanomortar mixture was then poured into the cone without tamping within 30 seconds. The cone was lifted vertically in one smooth motion to a height of approximately 30 cm, allowing the mixture to flow freely. The time required for the spread to reach a diameter of 500 mm ( $T_{500}$ ) was recorded using a stopwatch immediately after lifting. The maximum spread diameter was then measured in two perpendicular directions ( $D_1$  and  $D_2$ ), and their average was taken as the slump flow value [30]. After the test, the mixture was poured into pre-oiled cube molds (15 cm × 15 cm × 15 cm) and cured for 3, 7, and 28 days, as shown in [Figure 1](#). Density and compressive strength were subsequently measured at each curing age.



**Figure 1.** The manufacturing process of POFA-FA ECC nanomortar

### Density and compressive strength tests

Density is defined as the mass of a sample per unit volume. According to Ahmad Zawawi *et al.* [4], the recommended density range for oil palm shell lightweight aggregate concrete incorporating FA is between 1.60 and 2.50 g/cm<sup>3</sup>. The density test was carried out at the curing ages of 3, 7, and 28 days by measuring the mass and volume of each specimen. Before measurement, the specimens were surface-dried to remove excess moisture. The mass was determined using a digital balance, while the volume was calculated based on the specimen's dimensions. To determine the compressive strength of the ECC nanomortar, tests were conducted in accordance with SNI 1974:2011 using a Universal Testing Machine (UTM). The cube specimens (15 cm × 15 cm × 15 cm) were positioned centrally between the machine platens, and load was applied gradually at a uniform rate until failure occurred. The maximum load sustained by the specimen was recorded and used to calculate the compressive strength.

## Results and Discussion

### XRF analysis

Based on the results of the XRF test, the elemental and compound compositions of POFA and FA were determined. The chemical

compositions obtained from the XRF analysis are presented in Table 2. For POFA, the dominant oxides identified were SiO<sub>2</sub> = 48.9%, Fe<sub>2</sub>O<sub>3</sub> = 3.42%, and CaO = 13.8%. These values indicate that POFA possesses cementitious properties, as materials with a combined oxide content of SiO<sub>2</sub> + Al<sub>2</sub>O<sub>3</sub> + Fe<sub>2</sub>O<sub>3</sub> > 50% and CaO > 10% are classified as cementitious materials [31,32].

**Table 2.** Chemical composition of POFA and FA based on XRF analysis

Oxide compound	POFA (%)	FA (%)
Al <sub>2</sub> O <sub>3</sub>	-	14.0
SiO <sub>2</sub>	48.9	29.3
P <sub>2</sub> O <sub>5</sub>	6.97	-
SO <sub>3</sub>	4.10	-
K <sub>2</sub> O	20.5	1.14
CaO <sub>2</sub>	13.8	18.3
TiO <sub>2</sub>	0.15	1.54
V <sub>2</sub> O <sub>5</sub>	-	0.05
Cr <sub>2</sub> O <sub>3</sub>	-	0.07
MnO	0.61	0.65
Fe <sub>2</sub> O <sub>3</sub>	3.42	30.9
NiO	-	0.03
CuO	0.15	0.06
ZnO	0.06	0.02
SrO	0.12	0.48
MoO <sub>3</sub>	0.50	2.10
BaO	0.10	0.38
Eu <sub>2</sub> O <sub>3</sub>	0.10	0.38
Yb <sub>2</sub> O <sub>3</sub>	-	0.03
Re <sub>2</sub> O <sub>7</sub>	0.07	0.31

For FA, the chemical composition was  $\text{SiO}_2 = 29.3\%$ ,  $\text{Al}_2\text{O}_3 = 14\%$ ,  $\text{Fe}_2\text{O}_3 = 30.9\%$ , and  $\text{CaO} = 18.3\%$ . These values demonstrate that FA exhibits both cementitious and pozzolanic characteristics. According to established criteria, materials containing  $\text{SiO}_2 + \text{Al}_2\text{O}_3 + \text{Fe}_2\text{O}_3 > 50\%$  and  $\text{CaO} > 10\%$  are cementitious, while those with  $\text{SiO}_2 + \text{Al}_2\text{O}_3 + \text{Fe}_2\text{O}_3 > 70\%$  are classified as pozzolanic [12,32,33].

The high silica ( $\text{SiO}_2$ ) and alumina ( $\text{Al}_2\text{O}_3$ ) contents in both POFA and FA indicate strong potential for these materials to act as supplementary cementitious materials (SCMs) in ECC formulations, enhancing pozzolanic reactivity and contributing to improved mechanical and durability performance.

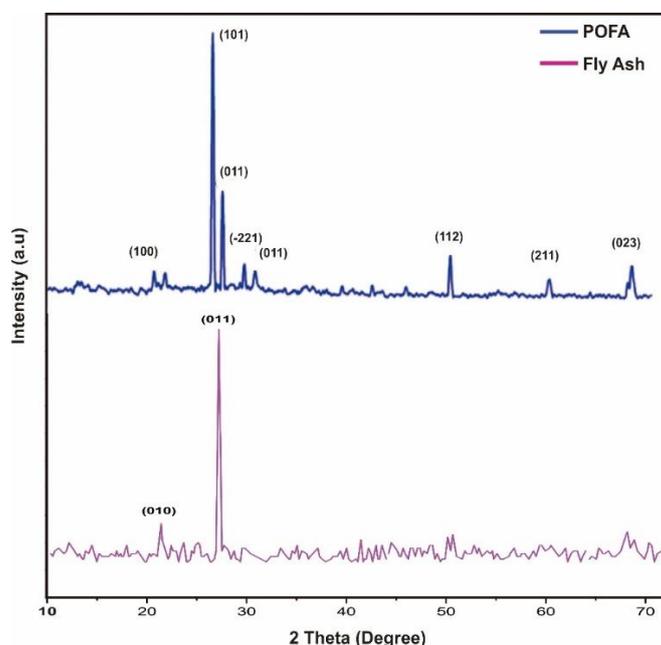
#### XRD analysis

The XRD analysis results confirmed that POFA and FA particles were successfully synthesized using the top-down method via ball milling, with average crystal sizes of 28.02 nm for POFA and 16.47 nm for FA (Table 3). Phase identification was conducted using the Match! - Phase

Identification from Powder Diffraction Data software and the Crystallography Open Database (COD). The identified crystalline phases of POFA and FA are presented in Figure 2 and Table 3.

As shown in Table 3, POFA contains two main crystalline phases:  $\text{SiO}_2$  and  $\text{Ca}_3(\text{Si}_3\text{O}_9)$ . The  $\text{SiO}_2$  phase exhibits a hexagonal crystal system with unit cell parameters  $a = 4.92 \text{ \AA}$ ,  $b = 4.92 \text{ \AA}$ , and  $c = 5.42 \text{ \AA}$ , showing diffraction peaks at  $2\theta = 20.72^\circ$ ,  $26.52^\circ$ ,  $50.07^\circ$ ,  $59.87^\circ$ , and  $68.04^\circ$ , consistent with previously reported POFA nanoparticles [34].

The  $\text{Ca}_3(\text{Si}_3\text{O}_9)$  phase exhibits a monoclinic crystal system with unit cell parameters  $a = 15.33 \text{ \AA}$ ,  $b = 7.28 \text{ \AA}$ , and  $c = 7.07 \text{ \AA}$ , showing peaks at  $2\theta = 27.50^\circ$  and  $29.65^\circ$ . The crystallite sizes, calculated from the FWHM, indicate nanoscale dimensions, confirming the nanoparticle nature of POFA. Meanwhile, FA nanoparticles exhibit a hexagonal crystal system with dominant  $\text{SiO}_2$  phase, characterized by diffraction peaks at  $2\theta = 21.37^\circ$  and  $27.17^\circ$ , and crystallite sizes around 16–17 nm. These XRD results confirm that both POFA and FA are suitable as nanosized cementitious materials.



**Figure 2.** XRD patterns of POFA and FA samples

**Table 3.** XRD phase identification data of POFA and FA

Sample	No.	Peak position ( $2\theta$ ) ( $^{\circ}$ )	FWHM ( $^{\circ}$ )	Crystallite size (D) (nm)	Miller indices (hkl)	Interplanar spacing (d) ( $\text{\AA}$ )
POFA	1	20.72	0.2255	35.82	100	4.27
	2	26.52	0.2288	35.67	101	3.36
	3	27.50	0.2105	38.85	011	3.25
	4	29.65	0.2286	35.94	-221	3.01
	5	30.73	0.3590	22.95	011	2.91
	6	50.07	0.2400	36.54	112	1.82
	7	59.87	0.3465	26.46	211	1.54
	8	68.04	0.4858	19.73	023	1.38
		<b>Average</b>		<b>28.02</b>		
FA	1	21.37	0.4819	16.78	010	4.15
	2	27.17	0.5061	16.15	011	3.28
			<b>Average</b>	<b>16.47</b>		

**Note:** Shape factor ( $K$ ) = 0.94; Wavelength ( $\lambda$ ) = 1.5406  $\text{\AA}$ .

The crystallinity of POFA and FA was also determined from the XRD analysis, as shown in Table 4.

Determining the proportion of amorphous and crystalline minerals in natural pozzolans is crucial for evaluating their oxide content. Previous studies have indicated that the amorphous phase represents the highly reactive alkali-active fraction. Although no general limit exists for the amorphous content in natural pozzolans, materials with less than 15% amorphous minerals have been shown to perform poorly as cement replacements [35].

Table 4 shows that POFA has a total area of 343.01 with a crystalline area of 96.81, corresponding to a crystallinity of approximately 28.22%, whereas FA has a total area of 500.83 with a crystalline area of 43.59, corresponding to a crystallinity of 8.70%. These values indicate that the amorphous fraction in both POFA and FA nanoparticles is significantly larger than the crystalline fraction. A higher proportion of amorphous content enhances compressive strength because the crystalline oxides in pozzolanic materials do not actively participate in pozzolanic reactions at room temperature.

Pozzolanic activity occurs when the active silica and alumina in the pozzolan react with calcium hydroxide (CH) produced during cement hydration, forming irreversible molecular bonds known as calcium silicate hydrate (C-S-H) [36].

The C-S-H gel, often referred to as tobermorite, crystallizes and contributes to cement strength, while unreacted CH does not enhance strength and may weaken the cement over time. Typically, CH constitutes about 20% of the cement weight, and in the worst case, it can lead to structural separation due to CH leaching [37].

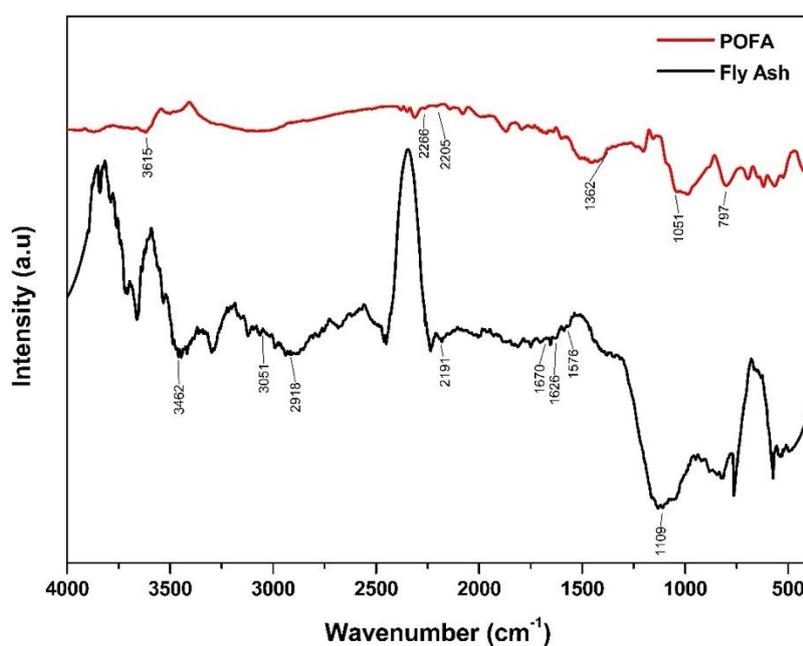
To mitigate this, additional silica-bearing pozzolanic materials such as POFA and FA are incorporated. In moist conditions, the pozzolan binds CH to form new C-S-H, enhancing concrete strength through the pozzolanic reaction [11,38].

#### FTIR analysis

The FTIR analysis was conducted to identify the functional groups present in POFA and FA. The results are presented in Figure 3. For POFA, characteristic peaks were observed at 797, 1,051, 1,362, 2,205, 2,266, and 3,615  $\text{cm}^{-1}$ .

**Table 4.** Crystallinity of POFA and FA

Sample	Crystal area	Total crystal area	Total area	Crystallinity (%)
POFA	4.94	96.81	343.01	28.22
	7.82			
	41.76			
	16.22			
	6.16			
	6.62			
	8.56			
	4.75			
FA	12.34	43.59	500.83	8.70
	31.26			

**Figure 3.** FTIR spectra of POFA and FA

The peak at  $1,362\text{ cm}^{-1}$  corresponds to C–H bonds in alkanes with strong intensity. The peak at  $797\text{ cm}^{-1}$  also indicates C–H bonds, this time in alkenes, with strong intensity. At  $1,051\text{ cm}^{-1}$ , a C–O bond characteristic of alcohols was observed with strong intensity. The peak at  $2,205\text{ cm}^{-1}$  corresponds to a triple bond  $\text{C}\equiv\text{C}$  in alkynes with variable intensity, while the peak at  $2,266\text{ cm}^{-1}$  indicates a  $\text{C}\equiv\text{N}$  triple bond in nitriles with strong

intensity. The peak at  $3,615\text{ cm}^{-1}$  is associated with N–H bonds in amines with medium intensity. These results align with previous studies, where the O–H stretching vibration of SiOH groups absorbs in the region around  $3,615\text{ cm}^{-1}$ , the asymmetric stretching vibration of Si–O is observed near  $1,051\text{ cm}^{-1}$ , and the  $797\text{ cm}^{-1}$  region corresponds to the crystalline quartz phase [39].

For FA, peaks were observed at 1,109, 1,576, 1,626, 1,670, 2,191, 2,918, 3,051, and 3,462  $\text{cm}^{-1}$ . The peak at 1,109  $\text{cm}^{-1}$  corresponds to C–O bonds in ethers with strong intensity. Peaks at 1,576  $\text{cm}^{-1}$  and 1,626  $\text{cm}^{-1}$  indicate C=C bonds in aromatic rings and alkenes, respectively, with variable intensity. The peak at 1,670  $\text{cm}^{-1}$  corresponds to C=O bonds in aldehydes with strong intensity. The peak at 2,191  $\text{cm}^{-1}$  indicates a C≡C triple bond in alkynes with variable intensity. Peaks at 2,918  $\text{cm}^{-1}$  and 3,051  $\text{cm}^{-1}$  correspond to C–H bonds in alkanes (strong) and alkenes (medium–strong), respectively. Finally, the peak at 3462  $\text{cm}^{-1}$  corresponds to N–H bonds in amines with medium intensity. The O–H stretching vibration of SiOH groups absorbs near 3,462  $\text{cm}^{-1}$ , while the Si–O–Si stretching vibration is observed around 1,109  $\text{cm}^{-1}$  [39]. These FTIR results confirm the presence of key functional groups that contribute to the pozzolanic and cementitious activity of both POFA and FA in ECC nanomortar.

#### Slump flow test

The slump flow test was conducted to evaluate the workability of fresh ECC mortar, which reflects the ease of handling and placement of the mixture, expressed as a measurable value. Slump is defined as the vertical drop at the center of the concrete surface, measured immediately after removing the slump mold [40]. The slump value is

influenced by the water-to-cement ratio ( $w/c$ ), which is the weight ratio of water to Portland cement in the concrete mix. A higher  $w/c$  ratio results in a higher slump, producing a more fluid mixture that is easier to work with. However, increasing the  $w/c$  ratio also reduces compressive strength, so it should not exceed 0.5 [41]. In this study, a  $w/c$  ratio of 0.3 was used for all sample variations. This relatively low ratio is expected to enhance the compressive strength of the POFA–FA ECC nanomortar. To improve workability while maintaining the target  $w/c$  ratio, superplasticizer was added. The superplasticizer allows the mixture to flow more freely, filling the pores within the composite and reducing porosity, which in turn increases the density of the nanomortar. The slump flow results of the POFA–FA ECC nanomortar are shown in Figure 4. As observed, increasing the percentages of POFA and FA led to a decrease in slump flow values. This is due to the water-absorbing properties of POFA and FA particles, which reduce the mixture's workability. This observation aligns with previous studies reporting decreased workability in concrete mixes with FA, attributed to the water absorption characteristics of FA [41].

Despite this reduction, all compositions remained within the standard slump flow range for high-quality concrete (500–800 mm), indicating that all POFA–FA ECC nanomortar mixtures are still highly workable and easy to handle [42].

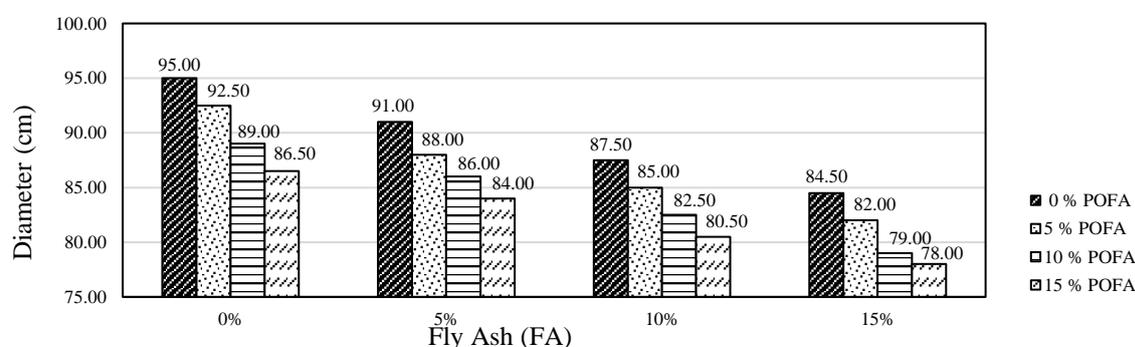


Figure 4. Slump flow test results of POFA–FA ECC nanomortar

Density test

Density is defined as the mass of a sample per unit volume and is also referred to as unit weight, specific gravity, or material compactness. The dry density of POFA-FA ECC nanomortar was measured to compare the effect of curing age at 3, 7, and 28 days. The results of the dry density tests are presented in Figure 5. As shown in Figure 5, the addition of POFA and FA reduces the density of the POFA-FA ECC nanomortar. Higher

percentages of POFA and FA result in lower density, which can be attributed to the fine particle size of the nanoparticles affecting the compactness of the mixture. According to SNI 1973:2008 and ASTM C567, the density of normal concrete ranges from 2,200 to 2,400 kg/m<sup>3</sup>. The densities observed in this study at 3, 7, and 28 days are lower than normal concrete but have not yet reached the typical range for lightweight concrete, which is 1,440–1,850 kg/m<sup>3</sup> [43,44].

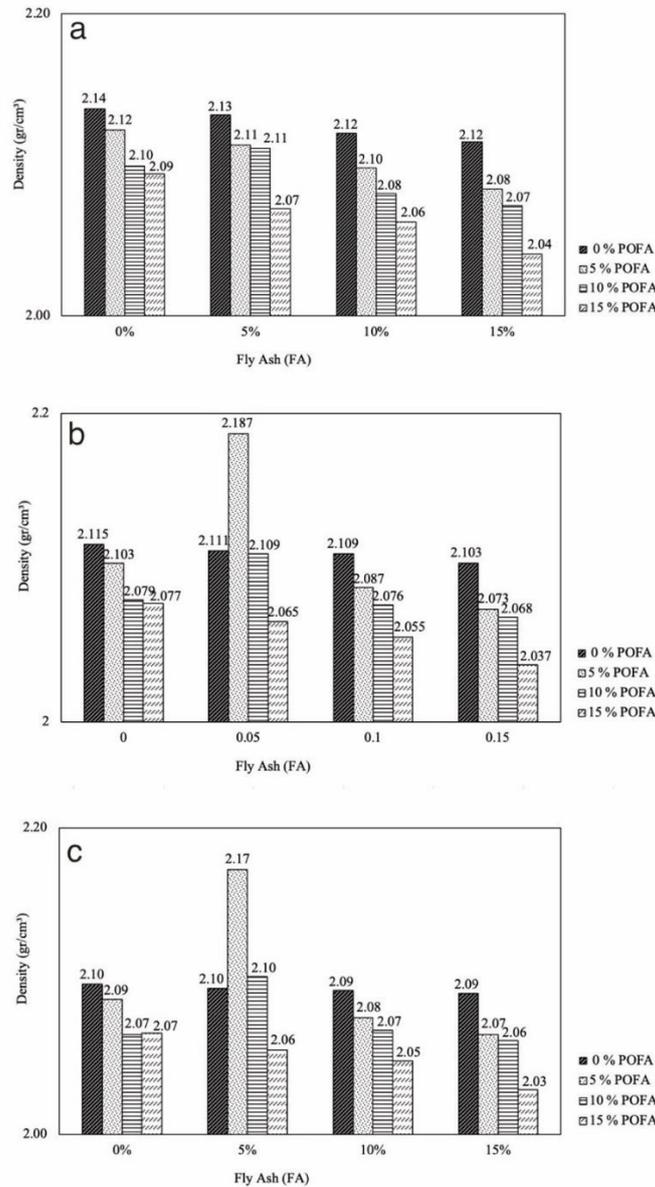


Figure 5. Dry density of POFA-FA ECC nanomortar at (a) 3 days, (b) 7 days, and (c) 28 days

Compressive strength

The compressive strength of POFA-FA ECC nanomortar was tested at 3, 7, and 28 days to evaluate the relationship between curing time and strength development. The results for 3 days, 7 days, and 28 days are presented in Figure 6.

At 3 days, the compressive strength ranged from 43.11 MPa to 58.09 MPa. After 7 days of curing, the strength increased to 45.21–65.78 MPa, and the maximum strength was observed at 28 days, reaching 46.34–72.73 MPa. This increase is attributed to the complete drying and hydration of the nanomortar at 28 days.

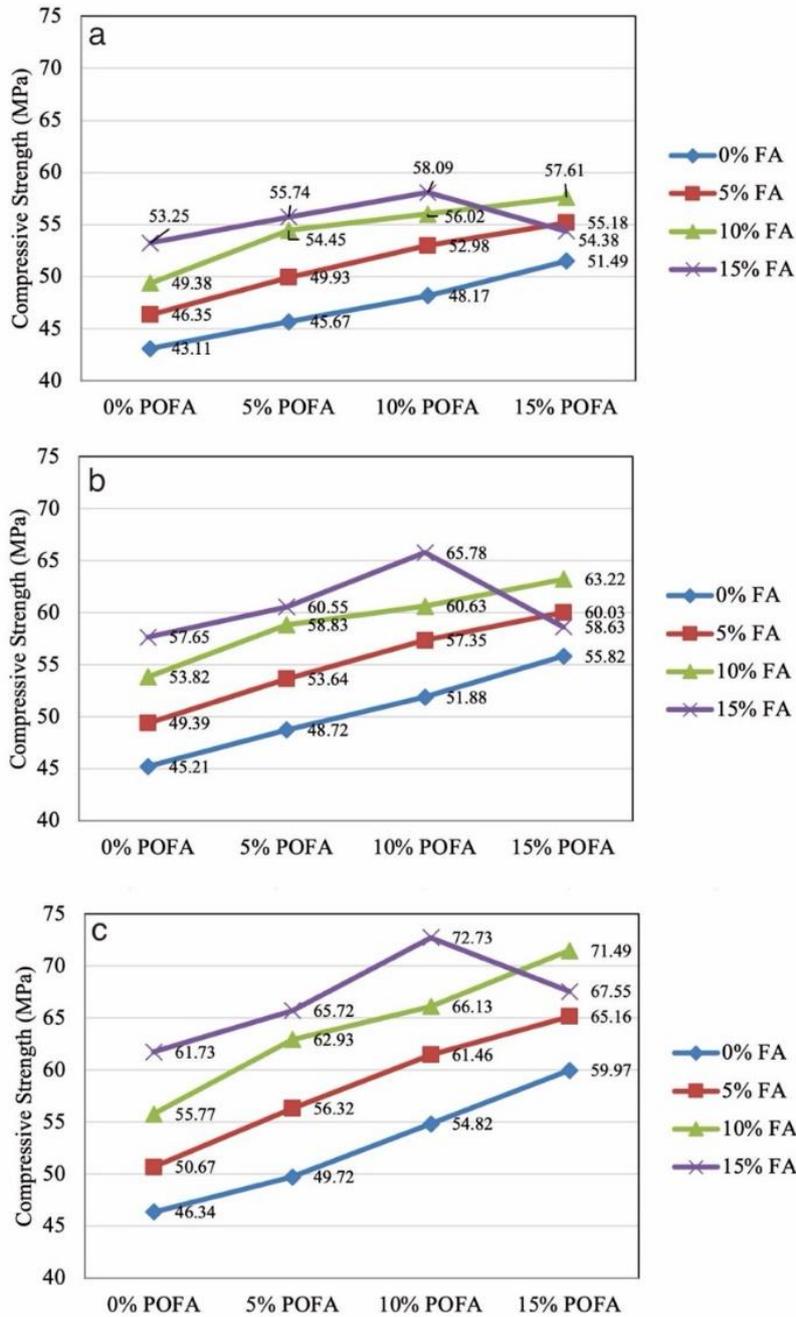


Figure 6. Compressive strength of POFA-FA ECC nanomortar at (a) 3, (b) 7, and (c) 28 days

Higher percentages of POFA and FA nanoparticles in the nanomortar corresponded to higher compressive strength. This indicates the contribution of  $\text{SiO}_2$ ,  $\text{Al}_2\text{O}_3$ ,  $\text{Fe}_2\text{O}_3$ , and  $\text{CaO}$  in POFA and FA as cementitious materials, which actively participate in the cement hydration process. According to SNI 03-1974:1990 and ASTM C 109, the compressive strength of normal concrete at 28 days ranges from 15 to 40 MPa [45]. The results confirm that all POFA-FA ECC variations exceeded the strength of normal concrete [46,47].

The highest compressive strength of 72.73 MPa was observed in the mix containing 10% POFA and 15% FA, using the same cement volume. Based on these results, all variations of POFA-FA ECC nanomortar qualify as high-strength concrete. High-strength concrete is defined as having a minimum compressive strength greater than 41.4 MPa [46] and is typically used in structural elements subjected to heavy loads, such as bridge girders, piers, pile foundations, sheet piles, and high-rise building structures. Using high-strength concrete allows for reduced structural dimensions, thereby lowering the overall weight of the structure. In practice, high-strength concrete often relies on finer and harder aggregates rather than large coarse aggregates to enhance strength and durability. In the production of high-strength concrete, the water-to-cement ratio (w/c) is carefully maintained between 0.2 and 0.3 to reduce porosity while preserving workability during placement, typically achieved through the addition of superplasticizers. Hydraulic cement hardens upon reacting with water. During this reaction, hydration occurs, releasing heat as the water-cement paste sets and binds the aggregate particles together. The incorporation of supplementary cementitious materials, such as POFA and FA, alongside Portland cement, enhances the workability of the concrete and reduces thermal cracking in massive structures by mitigating hydration heat. The silica ( $\text{SiO}_2$ )

present in POFA and FA reacts with  $\text{Ca}(\text{OH})_2$  to form calcium silicate hydrate (C-S-H), the primary compound responsible for concrete strength.  $\text{Ca}(\text{OH})_2$ , a product of cement hydration, is brittle and water-soluble; therefore, the presence of POFA and FA contributes to increased strength in POFA-FA ECC nanomortar.

#### Comparison with previous studies

As shown in Table 5, the POFA-FA composite achieved a maximum compressive strength of 72.7 MPa after 28 days, outperforming previously reported composites such as SPC15% + SS85% (60.5 MPa), HSSCC + SCM (50.4 MPa), and HSECC + DP 20% (44.1 MPa). This improvement is attributed to the enhanced pozzolanic activity of POFA and FA, which promotes additional formation of C-S-H gel and results in a denser microstructure.

**Table 5.** Comparison of compressive strength and density of POFA-FA nanomortar with previously reported cementitious composites

Composite	Age (days)	Compressive strength (MPa)	Density ( $\text{g}/\text{cm}^3$ )	Reference
Hollow glass microsphere	28	69.0	< 1.50	[48]
SPC15% + SS85%	28	60.5	Not reported	[49]
HSSCC + SCM	28	50.4	Not reported	[50]
HSECC + DP 20%	28	44.1	Not reported	[51]
POFA-FA ECC	28	72.7	1.45-1.85	This work

The density range of 1.45–1.85  $\text{g}/\text{cm}^3$  indicates that the POFA-FA composite can be classified as a lightweight, high-strength material, achieving an optimal balance between reduced unit weight and mechanical performance. These findings confirm

the effectiveness of the POFA–FA combination as a sustainable cementitious filler for producing high-strength, lightweight concrete materials.

## Conclusion

This study successfully developed and characterized POFA–FA nanomortar (ECC) with varying proportions of POFA and FA. Material characterization using XRF, XRD, and FTIR confirmed that POFA and FA are cementitious and pozzolanic, with POFA exhibiting hexagonal  $\text{SiO}_2$  and monoclinic  $\text{Ca}_3(\text{Si}_3\text{O}_9)$  phases, while FA showed a dominant hexagonal  $\text{SiO}_2$  phase. Both materials contained a significant amorphous fraction, enhancing their pozzolanic reactivity. Slump-flow tests indicated that all nanomortar mixtures maintained high workability within the standard range for self-compacting concrete (500–800 mm), although workability decreased slightly as the POFA and FA content increased due to water absorption by the nanoparticles. Dry density decreased with increasing nanoparticle content, ranging from 2.03 to 2.19  $\text{g}/\text{cm}^3$ , classifying the mixtures as lightweight concrete. Compressive strength increased with higher POFA and FA content, reaching a maximum of 72.7 MPa at 28 days, surpassing conventional and previously reported composites. This improvement is attributed to the pozzolanic reaction of  $\text{SiO}_2$ ,  $\text{Al}_2\text{O}_3$ , and  $\text{CaO}$  in POFA and FA, which promoted additional C–S–H gel formation, reduced porosity, and increased microstructural density.

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## Disclosure Statement

No potential conflict of interest was reported by the authors.

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